

MID-TERM PRACTICE QUESTIONS

CEE 361-513: Introduction to Finite Element Methods

Thursday Oct. 19

This are some example questions to sharpen your skills for the mid-term. In addition you should review the homework, precepts, and lecture notes.

PROBLEM 1

1. Let $d = 2$. $u = x_1x_2 + c$ be a scalar where c is any arbitrary constant. Find ∇u and $\nabla \cdot (\nabla u)$.
2. Let $d = 3$. $\mathbf{u} = x_1x_3\mathbf{e}_1 + x_2x_3\mathbf{e}_2$. Find the gradient of \mathbf{u} .
3. Is $\mathbf{T}(\mathbf{u}) = \sin(\mathbf{u} \cdot \mathbf{e}_1)\mathbf{e}_2 + \cos(\mathbf{u} \cdot \mathbf{e}_2)\mathbf{e}_1$ a tensor?
4. Let $\mathbf{x}_a = 2\mathbf{e}_1 + 5\mathbf{e}_2$ and $\mathbf{x}_b = 7\mathbf{e}_1 + 8\mathbf{e}_2$. Find the projection tensor that projects vectors along the direction $\mathbf{a} = \mathbf{x}_b - \mathbf{x}_a$.
5. Let $\{\mathbf{e}_i\}_{i=1}^3$ be a set of orthonormal basis. Let $\mathbf{a}, \mathbf{b}, \mathbf{v}$ be three vectors such that $\mathbf{b} = \mathbf{v} - \mathbf{v} \cdot \mathbf{a} \mathbf{a} / \|\mathbf{a}\|^2$. Show that \mathbf{a} and \mathbf{b} are linearly independent (i.e. $\alpha\mathbf{b} + \mathbf{a} = \mathbf{0} \Rightarrow \alpha = 0$).
6. Let $\text{tr}(\mathbf{A}) := \mathbf{A} : \mathbf{1}$ be the trace of a tensor \mathbf{A} . If $f = x_2x_3 + x_1x_3 + x_1x_2$, in which $\{\mathbf{e}_i\}_{i=1}^3$ is a set of orthonormal basis associated with the cartesian coordinates $\{x_i\}_{i=1}^3$. Show that $\nabla \cdot (\nabla f) = \text{tr}(\nabla(\nabla(f)))$.

PROBLEM 2

Consider the truss shown below. Foreach node we have associated coordinates \mathbf{q}_z and associated global degrees of freedom \mathbf{u}_z , where both \mathbf{q} and \mathbf{u} are vectors. All elements have the same E, A .

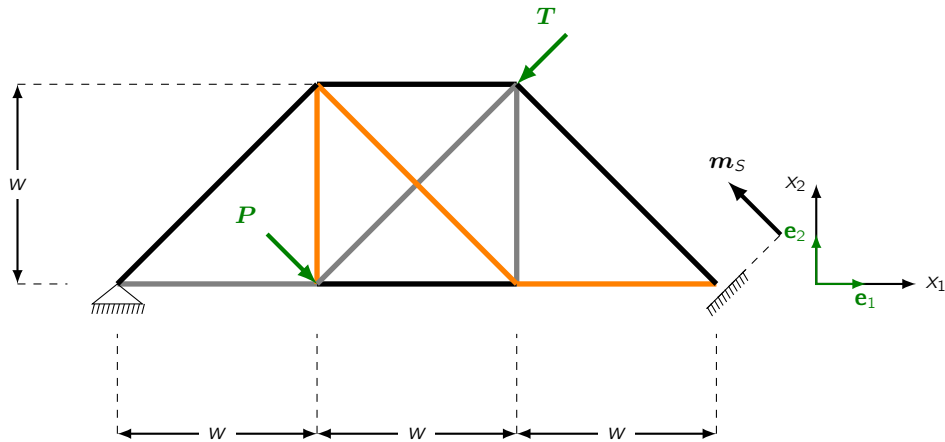


Figure 1: The system of uniaxial rods

1. Label each node and element and create a connectivity array.
2. For each element write the internal forces as the matrix vector operation of the *local element stiffness* and the *local degrees of freedom*.
3. For each element write the internal forces as the matrix vector operation of the *local element stiffness* and the *GLOBAL degrees of freedom* using the connectivity array.
4. For each node write the equilibrium equations in terms of the external forces, the reactions, and the internal forces.

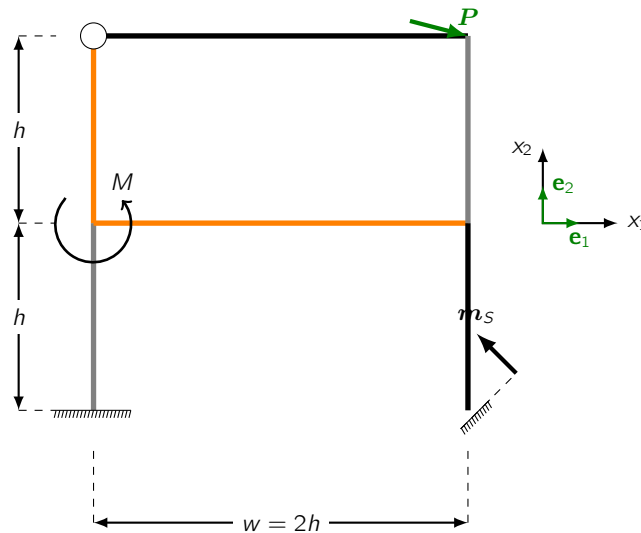
- Write down the equilibrium equations in matrix form. Namely, as we did in class, write the equilibrium equations with a load vector containing reactions and external forces, denoted it by $\{P\}$, the stiffness matrix denoted by $[K]$, and the vector of displacements $\{U\}$ such that

$$[K]\{U\} = \{P\}.$$

- At the leftmost node we prevent the truss from moving. At the rightmost node we allow the truss to move along a plane whose unit normal is \mathbf{m}_2 . Apply the aforementioned conditions to $[K], \{P\}$.
- What is the reaction force at the leftmost node?
- What is the reaction force at the rightmost nodes?

PROBLEM 3

Consider the frame shown below. At the lower- and left-most node we constrain the frame from moving in all directions and we prevent it from rotating. At the upper- and left-most node we have a hinge (hence no moment can be transferred). At the lowest- and right-most support the frame is allowed to move along a plane define by the normal \mathbf{m}_S . All elements have the same E, I, A .



- Label each element and node and write the connectivity array.
- For each node write the equilibrium equations in terms of the external force \mathbf{P} and moment M , and the internal forces $\mathbf{f}_{i,j}^e$ and moments $m_{i,j}^e$.
- Write the general expression of internal forces (and moments) as the matrix vector operation of the *local element stiffness* and the *local degrees of freedom*.
- For each element write the internal forces (and moments) as the matrix vector operation of the *local element stiffness* and the *GLOBAL degrees of freedom* using the connectivity array.
- Using $\mathbf{K}_{f_w}^e, \mathbf{k}_{f_\theta}^e, \dots$, write down the equilibrium equations in matrix form.
- At the lower- and left-most node we constrain the frame from moving in all directions and we prevent it from rotating. At the upper- and left-most node we have a hinge (hence no moment can be transferred). At the lowest- and right-most support the frame is allowed to move along a plane define by the normal \mathbf{m}_S . Apply the aforementioned conditions to the matrix form of the previous step.
- How would you determine the reactions?